

REC	CTORY:	TABLE	OF CON	TENTS:		-	NO.	REVISIOI	N DATE
SUZET 33RD	FE PIPER PLACE	ARCHITECTRU A 1.0	AL SITE PLAN & PR	OJECT NOTES			Δ	DATED 6/9/20 CITY CORRECTI	<u>)22</u> ONS 2022
ISLAND, FUNCTI	WA 98040 ON ARCHITECTURE	A 1.1	DETAILED SITE P SEDIMENT CONTR	LAN, TEMPOR	ARY EROSION &				_022
STLAKE WA 98	AVE N., SUITE 205	A 2.0 A 2.1 A 2.2	EXISTING BASEME MAIN FLOOR & F RASEMENT PLAN	ROOF DEMO PLA	AN 2/1 _AN 2/1				
2-9790 : JUDY	5	A 2.3 } A 3.0	MAIN FLOOR PLA WEST EXTERIOR	N Elev., Windov	SCHED., ROOF PLAN				0
ING: IEERING	INC.	A 3.1 A 3.2	EXTERIOR ELEVA SECTIONS, DOOR	TIONS SCHEDULE					810
ERSON WA 98	ST SUITE 302 109	A 3.3	SECTION B-B, S	TAIR SECTION	& DETAILS				<u>б</u>
5-4512 : BEN	2 McCANN	S 1.0 S 1.1	STRUCTURAL NO SHEARWALL SCHI	TES EDULE & DET.	AILS				M
ERING: SEO ASS	50C.	S 1.2 S 2.0	HOLDDOWN SHED BASEMENT LEVEL	ULE & DETAIL WALLS OVER	S FOUNDATION				
D, WA	2 W, SIE E 98036	S 2.1	MAIN LEVEL FRAN SHEARWALLS ROOF FRAMING F	MING PLAN O' PLAN OVER MA	VER BASEMENT LEVEL	s			
: DAVE	MATTHEWS	S 6.0 S 8.0	TYPICAL CONCRE MOMENT FRAME	TE DETAILS DETAILS		.0	\geq	2	Ś
GINEERI 35TH S	NG F, STW 200	S 8.1 S 8.2	MOMENT FRAME MOMENT FRAME	DETAILS DETAILS				\geq	02
WA 98 3-449 V KURT	409 1 DARCHER	S 8.3 S 9.0 S 9.1	MOMENT FRAME TYPICAL WOOD F	DETAILS RAMING DETA RAMING DETA	LS		\vdash		#2
	TAKOLEK	S 9.2 S 10.0	TYPICAL WOOD F TYPICAL COMPON	RAMING DETA IENTS	LS				Ż
		SURVEY							, L L
OTF	S	SURV	TOPOGRAPHIC SU	JRVEY					\triangleleft
:	REMODEL MAIN FLOOR KITO YARD COVERED DECK, NEW	CHEN, MOVE S V SIDE YARD N	TAIRS TO NEW RE	AR YARD ADI	DITION, NEW REAR T HOUSE TO		_		LAK 96
OR	GARAGE, NEW GUEST BATH	H, NEW MAIN F	LOOR MASTER SU	JITE, NEW ROO)F.				55T
	8429 SE 33RD PL MERCER	R ISLAND, WA	98040						372
	PARKRIDGE ADD, LOT 25 SF 9.6								80C 06.
	TYPE V B						105		
CAL	LANDSLIDE HAZARD, EROSI	ON CONTROL					1003	REGIST	IRED TECT
	19,302 SF (0.44 ACRES) FRONT YARD: 20' MIN								A I
	REAR YARD: 25' MIN SIDE YARD: 5' MIN, 18'	COMBINED (17	% OF LOT WIDTH:	106'-3")				JODITH A TO	SHETTON
	BASED ON LOT SLOPE. LO	W ELEVATION :	= 192.0' HIGHES	T ELEVATION	= 270.0'				
	30% - 50% LOT SLOPE AL	LOWS FOR 30	% LOT COVERAGE						
	EXIST ROOF = $NEW POOF =$		2659.6 SF						
	EXIST. DRIVEWAY = NEW COVERED PATIO/ DEC	CK =	2310.3 SF 413.4 SF	λ					
	EXIST. SHED ROOF = EXIST. SHED ROOF TO BE	REMOVED =	{289.5 SF -289.5 SF						
	TOTAL LOT COVERAGE= ALLOWABLE LOT COVERAGE		{5579.9_SF () 5790.6_SF ()	28.9%) 30%)				-	
	SEE SHEET A2.2 FOR IMP	ERVIOUS SURF	ACE A					, ∑,]
(533.3 SF			-		AND AND	212
Λ	EXIST WALKWAYS = EXIST. ROCKERIES/RETAININ	NG WALLS =	100.0 SF 75.0 SF	SEE SEE SEE	HEET A2.3)FOR NEW HARD SURFACE		MOI	331 ISL/	
ر م	EXIST. GRAVEL WALKWAY = NEW UNCOVERED PATIOS = EXIST. CONC. PATIO (TO B	E REMOVED) -	370.9 SF 490.9 SF					S R S R	NON.
<u>_1</u>	IOTAL HARDSCAPE AREA=		1036.8 SP (5	5.3%)				F29 ERCI	OJECT
	BASEMENT=		99.24 SF					$^7 \odot \Sigma ^7$) A
	EXISTING MAIN FLOOR= NEW MAIN FLR= EXISTING GARAGE=	1.	466.6 SF 439.7 SF 460.9 SF	$\sqrt{2}$			DATE	3/29	9/22
Z	MAIN FLOOR COVERED DEC TOTAL GROSS FLOOR AREA 40% OF 19 302-(7 720.8	K= 2 SE ALLOWED	2 <u>36.2 SF</u> 502.6 SF (18%)				DRAWN	I BY JT	SD
ON:	1456.1 (BSMT SF) X 61.7%	G (BELOW GRAE	DE % = 899.24	SF			CHECK	ED BY	
	A 2	<u>7.1</u>	$\frac{\text{COVERAGE}}{50.6} = \frac{1}{7}$	<u>RESULI</u> 15.2			CULLT		
	C 2	7.1	100	27.1			SITE	PIAN	
	TOTALS 16	51.6	100	99.8			PRC	J INFO	
	MAX 30' ABE (AVERAGE B (MID POINT FLEVATION X 1	UILDING ELEVA ENGTH OF WA	TION): LL) / TOTAL LEN	GTH OF WALL	SEGMENTS				
	(60,044.25) / 229= 262.2	2' ABE		JAN VI MALL			SHEET	NO.	
	AVERAGE GRADE (Aa)+(E A= 267.6 a= 64 . B= 262.7 b= 66	s⊳j+(Cc)+(Dd) 2 1	/a+b+c+d =				\wedge	1 ($\overline{}$
	$\begin{array}{c} c = 269.9 \\ D = 269.0 \\ \end{array} \qquad \begin{array}{c} c = 64. \\ d = 66. \\ \end{array}$	2 1					\square		
			(· · · · · · !	∎	,		-

(267.6X64.2)+(262.7X66.1)+(269.9X64.2)+(269.0X66.1)/64.2+66.1+64.2+66.1=267.3'AVERAGE EXISTING GRADE= 267.3', ALLOWABLE HT = 297.3'







EXISTING BASEMENT DEMO PLAN VERIFY ALL DIMENSIONS TO EXISTING ELEMENTS

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DIAGRAM OF NEW/REPLACED HARD SURFACE FOR STRUCTURES BASED ON SUPPÓRTED ROOF AREA BY NEW WALLS

NEW/REPLACED HARD SURFACE:

 $\mathbf{\hat{0}}$ NTS NORTH

NORTH BAY =	47.82 SF
WEST BAY =	28.38 SF
SOUTH/EAST ADDITION =	1493.38 SF
NEW CONCRETE PATIO =	369.90 SF
TOTAL=	1939.48 SF

TOTAL NEW/REPLACED HARD SURFACE IS UNDER THE 2000 SF THRESHOLD, THEREFORE, FULL DRAINAGE PLAN & REPORT BY A CIVIL ENGINEER IS NOT REQUIRED.

STAIR NOTES:

-	WIDTH	MIN. 36" CLEAR
_	TREAD DEPTH	10" CLEAR (ALLOW FOR 1" OVERHANG – 11" TOTAL)
_	RISER HEIGHT	7 $1/2" +/-$ VERIFY W/ EXISTING GRADE/ FLOOR HEIGHTS
		(MAX. HT. 7.75")
_	HANDRAIL	MIN. 34", MAX 38" ABOVE TREAD NOSINGS
_	HANDRAIL GRASP	MIN. 1-1/4", MAX. 2"
_	HANDRAIL PROJECTION	MAX. 4-1/2" FROM EACH SIDE OF STAIRWAY INTO
		REQUIRED WIDTH. MIN. $1-1/2$ " BETWEEN THE WALL
_	GUARDRAIL	MAX OPENING 4" PER SRC R312.1.3
		MIN. UNIFORMLY DISTRIBUTED LIVE LOADS (LBS PER SF)
		200LB CONCENTRATED LOAD ON THE TOP RAIL & 50 PSF
		ON GUARDRAIL INFILL COMPONENTS PER SRC R301.5-
		SEE STRUCTURAL FOR DETAILS

NOTES:

- 1. CONTRACTOR WILL APPLY FOR PLUMBING, MECHANICAL, ELECTRICAL PERMITS SEPARATELY. CONTRACTOR TO REVIEW EXISTING CONDITIONS PRIOR TO STARTING CONSTRUCTION.
- 2. ALL WALLS THAT HAVE FINISHES REMOVED WILL BE SUBJECT TO CURRENT CODE RQMTS - INCLUDES PLUMBING/MECHANICAL/ELECTRICAL/INSULATION.
- 3. WHOLE HOUSE FAN SHALL BE LOCATED/ASSOCIATED WITH THE MAIN FLOOR GUEST BATH, THIS FAN TO BE EQUIPPED WITH CONTROLS CAPABLE OF MANUAL AND AUTOMATIC OPERATION, SUCH AS A CLOCK TIMER AND SHALL BE DESIGNED TO RUN CONTINUOUSLY PER SRC M1507.3.3.
- 3.1. IN NEW MASTER BATH & CLOSET- INSTALL PANASONIC WHISPERQUIET FAN SIZED PER SPACE. (75 C.F.M., 1.0 SONES OR BETTER). FAN SHALL TERMINATE VERTICALLY TO THE EXTERIOR OF THE HOUSE. 3.2. NEW KITCHEN HOOD ON MAIN FLOOR TO BE SELECTED- MIN 100 C.F.M.,
- 1.5 SONES OR BETTER. FANS SHALL TERMINATE HORIZONTALLY TO THE EXTERIOR OF THE HOUSE. 4. DOOR JAMBS SHALL BE 3 $\frac{1}{2}$ " TYPICAL, UNLESS NOTED OTHERWISE.
- 5. SEE SHTS A3.0 & A3.2 FOR WINDOW & DOOR SCHEDULES AND ELEVATIONS
- (A3.0 & A3.1) FOR ADDITIONAL INFORMATION.
- 6. PROVIDE BLOCKING FOR FUTURE GRAB BARS IN BOTH MAIN FLOOR BATHROOMS.

 \bigcirc 2022 FORM \perp FUNCTION

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WINDOW SIZE	OPERATION	MATERIAL	MFGR	GLAZING	U-VALUE	NOTES	
5'-9" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
5'-10" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
3'-10" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #8	
3'-10" x 4'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #9	
5'-6" x 4'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
5'-6" x 4'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
3'-0" x 2'-4"	AWNING	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, (E) OPENING	
3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #3	
3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED MULLED WITH #4	
3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED	
3'-10" x 10'-8"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TEMPERED	
5'-6" x 6'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
6'-0" x 6'-2"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON, EGRESS	
4'-0" x 6'-2"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
		THERMAL BREAK					\mathcal{T}
$3 = 0 \times 2 = 2$		WOOD CLAD	MANYIN SIGNATURE MODERN	LV# L, VLN	U.Z/ MIIN		_)
<u> </u>	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	=) _
<u> </u>	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON, TRANSOM	IJ
6'-0" x 6'-0"	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, OBSCURE	0.27 MIN	ARGON, TEMPERED SANDBLASTED	
1'-4" x 6'-0"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, OBSCURE	0.28 MIN	ARGON, TEMPERED SANDBLASTED	
		THERMAL BREAK					$\overline{}$
0 = 0		WOOD CLAD	MARVIN SIGNATURE MUDERN		0.2/ MIN	$\xrightarrow{\text{ARGUN, IRANSUM}}$	
4'-11" x 6'-0"	CSMT	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.28 MIN	ARGON	
5'-4" x 1'-6"	FIXED	THERMAL BREAK	MARVIN SIGNATURE MODERN	LOW-E, CLR	0.27 MIN	ARGON	
7' = 6'' + 1' + 0''		THERMAL BREAK					7
		WOOD CLAD					_{
<u> </u>	FIXED	THERMAL BREAK WOOD CLAD	MARVIN SIGNATURE MODERN	LOW=E, CLR	0.27 MIN	ARGON, TRANSOM	_ ^ک
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MARK	WINDOW SIZE	OPERATION	MATERIAL	MFGR	GLAZING	U-VALUE	NOTES
25	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
26	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
27	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
28	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
29	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
30	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
31	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON
32	2'-0" x 4'-0"	SKYLIGHT	ALUMINUM	VELUX	LOW-E, CLR	0.50	ARGON

LEGEND

EXISTING TO REMAIN

	EXISTING TO BE DEMOLISHED	د	_	
	NEW WALLS			NEW ASPHALT TAB ROOF
<u>S:</u>	MIN. 36" CLEAR 10" CLEAR (ALLOW FOR 1" OVERHANG – 11" TOTAL) 7 1/2" +/- VERIFY W/ EXISTING GRADE/ FLOOR HEIGHTS –	21'-4½"	10,	NEW DECK & GUARDRAIL (TO BE SELECTED)
P ECTION	(MAX. HT. 7.75") MIN. 34", MAX 38" ABOVE TREAD NOSINGS MIN. 1–1/4", MAX. 2" MAX. 4–1/2" FROM EACH SIDE OF STAIRWAY INTO REQUIRED WIDTH. MIN. 1–1/2" BETWEEN THE WALL MAX OPENING 4" PER SRC R312.1.3 MIN. UNIFORMLY DISTRIBUTED LIVE LOADS (LBS PER SF)		8'-4"	LINE OF (E) ROCKER VERIFY LOCATION & C CONFLICTS W STRUCT
	200LB CONCENTRATED LOAD ON THE TOP RAIL & 50 PSF ON GUARDRAIL INFILL COMPONENTS PER SRC R301.5– SEE STRUCTURAL FOR DETAILS			FΔ<

STAIR NOTES:

- WIDTH
- TREAD DEPTH
- RISER HEIGHT
- HANDRAIL
- HANDRAIL GRASP
- HANDRAIL PROJECTIO

– GUARDRAIL

DOOR SCHEDULE- BASEMENT & MAIN FLOOR

MARK	DOOR SIZE W X H	OPERATION	MATERIAL	GLAZING	U-VALUE	NOTES
A	3'-0" x 6'-8"	SWING	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY)
В	3'-0" x 6'-8"	SWING	SC WOOD	NA	NA	PRIVACY LATCH
С	2'-0" x 9'-0"	SWING/ FRENCH DR	SC WOOD OR GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.20 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL- COORD. ASTRIGAL OPTIONS W/ ARCHITECT
D	4'-0" x 9'-0"	SWING/ FRENCH DR	SC WOOD OR GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.20 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY)
E	3'-0" x 6'-8"	POCKET	SC WOOD	NA	NA	
F	2'-8" x 6'-8"	SWING	SC WOOD	NA	NA	PRIVACY LATCH
G	3'-0" x 6'-8"	SWING	SC WOOD	NA	NA	PRIVACY LATCH
н	2'-10" x 6'-8"	SWING	SC WOOD	NA	NA	PRIVACY LATCH
	NOT USED FOR	CLARITY				
J	2'-10" x 6'-8"	POCKET	SC WOOD	NA	NA	PRIVACY LATCH
К	2'-10" x 6'-8"	POCKET	SC WOOD	NA	NA	PRIVACY LATCH
L	2'-10" x 6'-8"	POCKET	SC WOOD	NA	NA	PRIVACY LATCH
М	2'-8" x 6'-8"	POCKET	SC WOOD	NA	NA	PRIVACY LATCH
N	2'-8" x 6'-8"	POCKET	SC WOOD	NA	NA	
0	NOT USED FOR	CLARITY				
P	3'-0" x 6'-8"	SWING	SC WOOD	NA	NA	
Q	3'-0" x 6'-8"	SWING	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL
R	2'-6" x 9'-0"	FIXED/ FRENCH DR	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL
S	(2) 2'-6" x 9'-0"	SWING/ FRENCH DR	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL
Т	3'-8" x 9'-0"	FIXED/ FRENCH DR	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL
U	(6) 3'-0" x 9'-0"	SLIDER/ FRENCH DR	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL
V	3'-0" x 9'-0"	SWING/ FRENCH DR	GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY)
W	16'-0" x 8'-0"	GARAGE	OBSCURE GLASS/ SC WOOD	LOW E/ARGON TEMPERED	0.30 MIN	THRESHOLD BY MFGR W/ WEATHERSTRIPPING FOR TIGHT SEAL
X	2'-6" x 6'-0"	SHOWER	GLASS	NA	NA	TEMPERED GLASS
Y	3'-0" x 6'-8"	SWING	SC WOOD	NA	NA	20 MIN RATED DR ON CLOSER ALUMINUM THRESHOLD BY PEMCO OR EQUAL SMOKE GASKETING FOR A TIGHT SEAL KEYED LOCK & DEADBOLT (MATCH HOUSE KEY)

DOOR GENERAL NOTES:

1. ALL DOORS TO BE NFRC CERTIFIED.

2. CONTRACTOR TO CONFIRM ROUGH OPENING REQUIREMENT W/ MNF'R

3. ALL INTERIOR & EXTERIOR DOORS BY LOEWEN OR SIMPSON OR EQUIVALENT.

4. SET EXTERIOR DOORS IN DOOR PAN PER NORTHWEST WALL & CLG BUREAU STANDARD DETAILS

- 5. ALL EXT. DOOR OPENINGS TO BE WRAPPED W/ VIDAFLEX F OR APPROVED EQUAL PEEL & STICK OR
- METAL FLASHINGS PER THE NORTHWEST WALL & CLG BUREAU STANDARD DETAILS
- 6. ALL U-VALUES PROVIDED FOR DOORS ARE PRESCRIPTIVE VALUES (MINIMUMS TO BE USED) UNTIL

SPECIFIC MANUFACTIRERS/DOOR MODELS ARE SELECTED.

7. ALL HARDWARE TO BE LEVER TYPE- FINISH TO BE SELECTED.

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01000 - GENERAL REQUIREMENTS

THE STRUCTURAL NOTES SUPPLEMENT THE PLANS AND SPECIFICATIONS. ANY DISCREPANCY FOUND BETWEEN THE DRAWINGS, NOTES, SPECIFICATIONS, SITE CONDITIONS, AND ARCHITECTURAL PLANS SHALL BE REPORTED TO THE ARCHITECT WHO SHALL CORRECT THE DISCREPANCY IN WRITING. ANY WORK COMPLETED AFTER DISCOVERY OF THE DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S RISK. REFER TO ARCHITECTURAL PLANS FOR OPENINGS, ARCHITECTURAL TREATMENTS, AND DIMENSIONS NOT SHOWN. CONSULT MECHANICAL PLANS FOR DUCTS AND PIPES ETC. NOT SHOWN.

THE CONTRACTOR SHALL PROVIDE BRACING AND SUPPORT REQUIRED FOR TEMPORARY CONSTRUCTION LOADS AND FOR STRUCTURAL COMPONENTS AS REQUIRED DURING ERECTION. BACKFILL BEHIND WALLS SHALL NOT BE PLACED UNTIL THE WALLS ARE PROPERLY SUPPORTED.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATION OF ALL WORK INCLUDING BUT NOT LIMITED TO EXCAVATION, SHORING, AND OTHER WORK WITH ALL UTILITIES AND ADJACENT PROPERTIES. CALL THE UTILITY LOCATE SERVICE PRIOR TO ANY WORK AT 1-800-424-5555.

01001 - CODE REQUIREMENTS

ALL DESIGN AND CONSTRUCTION SHALL CONFORM TO THE 2018 INTERNATIONAL BUILDING CODE AS ADOPTED BY SEATTLE, WASHINGTON.

01100 - DESIGN LOADS DEAD LOADS:

ACTUAL WEIGHT OF MATERIALS OF CONSTRUCTION AND PERMANENT EQUIPMENT.

FLOOR LIVE LOADS: FLOORS (RESIDENTIAL) DECKS		40 PSF 60 PSF		
ROOF LIVE LOADS:				
ROOF		20 PSF		
SNOW LOAD DESIGN DATA: Pg = 20 PSF, Pf = 20 PSF, 0	Ce = 0.9, ls = 1.0, Ct = 1.0,	25 PSF UNIFOR	RM	
WIND DESIGN DATA: BASIC WIND SPEED WIND IMPORTANCE FACT WIND EXPOSURE TOPOGRAPHICAL FACTOI INTERNAL PRESSURE CO COMPONENT/CLADDING	ror R Defficient Wind Pressure	100 MPH (3-SE lw = 1.0 EXPOSURE B Kzt = 1.3 GCpi = +/- 0.18 P(C) = 25 PSF	COND GUST)	
EARTHQUAKE DESIGN DATA SEISMIC IMPORTANCE FA OCCUPANCY CATEGORY SPECTRAL RESPONSE AC SITE CLASS SPECTRAL RESPONSE CO SEISMIC DESIGN CATEGO	A: ACTOR CCELERATIONS OEFFICIENTS DRY	le = 1.0 II Ss = 1.401 D SDS = 0.934 D	S1 = 0.487 SD1 = 0.584	
(E-W) EQUIVELENT LATER WALL SYSTEM W/LIG WALLS SHEATHED W STRUCTURAL PANEL SHEAR RESISTANCE T = 0.189 (LESS THAN PER ASCE 7-17 SECT	RAL FORCE - BEARING GHT FRAMED WOOD VITH WOOD LS RATED FOR E N 1.5*Ts) THEREFORE NO TION 11.4.8 EXCEPTION #	R = 6.5 SITE RESPONS 2.	Cs = 0.14 SE ANALYSIS REQUIRED	
(N-S) COMBINATION OF FF IN THE SAME DIREC R = 3.5 ORDINARY M	RAMING SYSTEMS TION (ASCE 7-16 12.2.3) R OMENT FRAME. GOVERN	R = 3.5 2 = 6.5 WOOD FR NNG R VALUE TI	Cs = 0.27 RAMED SHEAR WALLS, HIS DIRECTION = 3.5.	
01200 - GEOTECNICAL INVES FOUNDATION DESIGN BASE REPORT DATED FEBRUARY FOUNDATION CONSTRUCTIO CERTIFIED INSPECTOR WITH REFUSAL CRITERIA. FILLS TO INSTALLED IN LIFTS NO GRE INTERIOR AND EXTERIOR SI ASTM D-1557.	STIGATION D ON REPORT NO. 2537.0 28, 2022 PREPARED BY 2 ON TO BE PERFORMED F H LOG CONFIRMING EAC O BE COMPACTED TO 95 EATER THEN 10 INCHES. A LABS ON GRADE SHALL E	D1 DATED JANU, ZIPPER GEO AL PER REPORT. AL H PILE DRIVEN I MODIFIED PR A MINIMUM OF 1 BE COMPACTED	ARY 28, 2022, AND SUPPLEMENTAL LL SITE PREPARATION AND LL PILE DRIVING TO BE INSPECTED E IN ACCORDANCE WITH SOILS REPO ROCTOR PER ASTM D-1557, AND 12 INCHES OF SOIL UNDER NEW O TO 95% MODIFIED PROCTOR PER	3Y A RT
ALL FOUNDATIONS SHALL B SOILS OR ABOVE PROPERLY STRENGTH PLACED ABOVE ENGINEER. WHERE FOUNDA NET BEARING CAPACITY OF	E FOUNDED ON PIPE PIL Y COMPACTED STRUCTU ADEQUATE NATIVE SOIL ATIONS ARE FOUNDED A 2000 PSF HAS BEEN USE	E OR BY ON AT JRAL FILL OR CE S PER THE DISC TOP CONDITION ED FOR DESIGN	LEAST MEDIUM DENSE / STIFF NATI DF WITH 100 PSI COMPRESSIVE CRESSION OF THE GEOTECHNICAL NS DESCRIBED ABOVE, AN ALLOWAE I.	VE BLE
GEOTECHNICAL DESIGN PA	RAMETERS HAVE BEEN (COORDINATED	WITH ZIPPER GEO AS LISTED BELOW	N
DESIGN PARAMETERS ARE	AS FOLLOWS:			

PASSIVE EARTH PRESSURE	400 PCF (ULTIMATE)
COEFFICIENT OF FRICTION	0.5 (ULTIMATE)
SOIL PROFILE	SITE CLASS D

ALL FOUNDATION INSTALLLATIONS SHALL BE SUBJECT TO APPROVAL OF THE GEOTECHNICAL ENGINEER.

PIPE PILE: INSTALLATION REQUIREMENTS:

TWO AND THREE INCH DIAMETER PIPE PILE SHALL CONSIST OF PIPE PER ASTM A53 GRADE B AND BE DRIVEN AT LEAST 10 FEET INTO COMPETENT SOIL. PIPE PILE REACHING THE FOLLOWING PENETRATION RATES MAY BE ASSIGNED THE FOLLOWING COMPRESSIVE CAPACITIES. PIPE PILE SHALL BE INSTALLED USING A HYDRAULIC IMPACT HAMMER CARRIED ON LOADS THAT ALLOW THE HAMMER TO SIT ON THE TOP OF THE PILE DURING DRIVING. IF ALTERNATE DRIVING MEATHODS ARE USED, COORDINATE REQUIRED LOAD TESTS WITH GEOTECHNICAL ENGINEER. GEOTECHNICAL SPECAIL INSPECTOR SHALL BE CONTINOUSLY PRESENT DURING PIPE PILE INSTALLATION. (F.D.R. - FINAL DRIVING RATE):

PILE DIAMETER	F.D.R. 90 LB. PERCUSSION DRIVER	F.D.R. 650 LB PERCUSSION DRIVER	ALLOWABLE COMP. CAPACITY
2 INCH (X-STRONG) SCHEDULE 80	60 SEC/INCH	NA	3 TONS (6,000 LB)
3 INCH (STANDARD)	NA	15 SEC/INCH	6 TONS (12,000 LB)
(E) 4 INCH	NA	NA	10 TONS (20,000 LB)

FIELD TESTING REQUIREMENTS:

LOAD TESTS ARE NOT REQUIRED FOR TWO OR THREE INCH DIAMETER PIPE PILES THAT ARE DRIVEN IN ACCORDANCE WITH THE RECOMMENDATIONS PRESENTED IN JANUARY 28, 2022 REPORT PREPARED BY ZIPPER GEO, AND PROVIDED THAT A ZGA REPRESENTATIVE OBSERVES INSTALLATION OF THE PILES AND VERIFIES THAT REFUSAL HAS BEEN ACHIEVED.

01300 - SHOP DRAWING SUBMITTAL PROCESS

SHOP DRAWINGS ARE TO BE SUBMITTED TO THE ARCHITECT AND ENGINEER OF RECORD FOR APPROVAL PRIOR TO FABRICATION. IF SHOP DRAWINGS DIFFER FROM THE APPROVED DESIGN DRAWINGS, NEW DESIGN DRAWINGS BEARING THE SEAL AND SIGNATURE OF A LICENSED STATE OF WASHINGTON STRUCTURAL ENGINEER SHALL BE SUBMITTED ALONG WITH THE SHOP DRAWINGS TO THE BUILDING OFFICIAL FOR APPROVAL PRIOR TO FABRICATION.

SHOP DRAWINGS ARE REQUIRED FOR STRUCTURAL STEEL AND PROPRIETARY GUARD COMPONENT.

01400 - INSPECTIONS AND SPECIAL INSPECTIONS THE CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE ALL INSPECTIONS REQUIRED BY THE LOCAL BUILDING DEPARTMENT.

SPECIAL INSPECTIONS ARE NOT REQUIRED FOR GROUP R-3 OCCUPANCIES UNLESS OTHERWISE REQUIRED BY THE BUILDING OFFICIAL.

01600 - QUALITY ASSURANCE REQUIREMENTS

THE QUALITY ASSURANCE PLAN SHALL BE TO VERIFY THAT THE SPECIAL INSPECTIONS NOTED IN SECTION 01400 AND THE STRUCTURAL OBSERVATION NOTED IN SECTION 01500 HAVE BEEN COMPLETED AND THAT SUPPORTING DOCUMENTATION NOTED IN SUCH SECTIONS HAS BEEN PROVIDED.

QUALITY ASSURANCE PLAN IS NOT REQUIRED FOR STRUCTURES OF LIGHT WOOD FRAMING WITH DESIGN SPECTRAL RESPONSE AT SHORT PERIODS, SDS, NOT EXCEEDING 0.50g.

QUALITY ASSURANCE PLAN IS NOT REQUIRED FOR WIND EXPOSURE B WHERE BASIC WIND SPEED IS LESS THAN 120 MPH.

SUMMARY: A QUALITY ASSURANCE PLAN IS NOT REQUIRED BY CODE FOR THIS STRUCTURE.

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01700 - EXECUTION REQUIREMENTS INSTALLATION OF ALL STRUCTURAL COMPONENTS SHALL BE AS REQUIRED PER ALL LOCAL

02000: SITE CONSTRUCTION ALL SITE CONSTRUCTION SHALL BE CONSISTENT WITH THE GEOTECHNICAL ENGINEERING RECOMMENDATIONS AS NOTED IN THE GEOTECHNICAL ENGINEERING REPORT (SEE SECTI AND IN SUBSEQUENT DIRECTIVES.

02100 - EXCAVATION SUPPORT AND PROTECTION

EXCAVATION FOR FOUNDATIONS SHALL BE PER PLAN DOWN TO UNDISTURBED NATIVE MA THE GEOTECHNICAL ENGINEERING RECOMMENDATIONS. OVER-EXCAVATED AREAS SHALL BACKFILLED WITH LEAN CONCRETE OR PER GEOTECHNICAL RECOMMENDATIONS AT THE CONTRACTOR'S EXPENSE.

EXCAVATION SLOPES SHALL BE SAFE AND SHALL NOT BE GREATER THAN THE LIMITS SPEC LOCAL, STATE, AND NATIONAL SAFETY REGULATIONS.

INSTALLATION OF CONSTRUCTION SHORING, IF REQUIRED, SHALL BE PER THE SHORING D NOTES, AND SPECIFICATIONS.

02200 - BACKFILL AND COMPACTION

BACKFILL SHALL NOT BE PLACED UNTIL THE REMOVAL OF FORMWORK AND OF ANY DEBRI BEHIND ALL WALLS SHALL NOT BE PLACED UNTIL THE WALLS ARE PROPERLY SUPPORTED MATERIAL AND PLACEMENT PROCEDURES SHALL BE CONSISTENT WITH THE GEOTECHNIC RECOMMENDATIONS.

03000 - CAST-IN-PLACE CONCRETE

CONCRETE CONSTRUCTION SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTE ST 318-14 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

CEMENT AND CONCRETE SHALL CONFORM TO IBC SECTION 1903. ADMIXTURES SHALL BE THE ENGINEER OF RECORD AND SHALL COMPLY WITH ACI 318-14 SECTION 3.6. CONCRETE EXPOSED TO FREEZING AND THAWING SHALL HAVE AN AIR ENTRA ADMIXTURE CONFORMING TO IBC SECTION 1904.2. THE USE OF WATER SOLUBLE CHLORID NOT BE USED.

CONCRETE MIX DESIGNS SHALL MEET THE FOLLOWING REQUIREMENTS:	
(1) 28 DAY MAX. STRENGTH fc [PSI] (2) MAX. WATER / CEMENT RATIO (3) MAX	. SLUMP [IN] (4
ENTRAINMENT [%] (5) SPECIAL INSPECTION REQUIRED (6) MIN. 90 LB SACKS	OF CEMENT (7
APPLICATION.	

(1)	(2)	(3)	(4)	(5)	(6)	(7)
3000	0.45	à+/-1	5 + /-1	ŇÓ	()	ÉXTERIOR SLAB ON GRADE
3000	0.41	4+/-1	0+/-1	NO		INTERIOR SLAB ON GRADE
3000	0.50	5+/-1	0+/-1	NO		FOOTINGS AND GRADE BEAM
3000	0.45	5+/-1	5+/-1	NO		STEMS
3000	0.50	5+/-1	5+/-1	NO		ALL OTHER CONCRETE

SPECIAL INSPECTION IS NOT REQUIRED AS THE DESIGN IS BASED ON fc = 2500 PSI. CHAMFER ALL EXPOSED CORNERS PER THE ARCHITECTURAL PLANS OR 3/4 INCH IF NOT S THE ARCHITECT.

03100 - REINFORCING STEEL

REINFORCING STEEL DETAILING, FABRICATION, AND PLACEMENT SHALL BE PER ACI 318-14 STEEL SHALL MEET THE FOLLOWING REQUIREMENTS:

ASTM A-615 DEFORMED BARS GRADE 40 (fy=40 KSI) FOR #3 BARS ONLY ASTM A-615 DEFORMED BARS GRADE 60 (fy=60 KSI) FOR #4 BARS AND LARGER ASTM A-706 DEFORMED BARS GRADE 60 (fy=60 KSI) FOR ALL WELDABLE BARS ASTM A-1064 SMOOTH BAR (fy=60 KSI) FOR WELDED WIRE FABRIC

REINFORCING FOR SLABS ON GRADE SHALL BE 6X6 W1.4XW1.4 WELDED WIRE FABRIC OR I UNLESS NOTED OTHERWISE. PROVIDE LAP SPLICES PER THE LAP SPLICE SCHEDULE ON S REINFORCING STEEL AT ALL WALLS, SLABS, AND FOOTINGS SHALL BE CONTINUOUS AROU ELSE CORNER BARS SHALL BE PROVIDED.

COVER REQUIREMENTS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:

CONCRETE CAST AGAINST EARTH

- ALL BAR SIZES3" FORMED SURFACE EXPOSED TO EARTH OR WEATHER
- #6 AND LARGER . .
- #5 AND SMALLER . . .1 1/2" CONCRETE NOT EXPOSED TO EARTH OR WEATHER
- WALLS AND JOISTS . .1 1/2"
- #14 AND #18 BARS . .
- SLABS AND JOISTS
- #14 AND #18 BARS1 1/2"
- #11 BARS AND SMALLER 1" BEAMS, COLUMNS
- PRIMARY REINFORCEMENT1 1/2"
- TIES, STIRRUPS, AND SPIRALS ... 1 1/2"

REINFORCING STEEL SHALL BE ACCURATELY PLACED AND ADEQUATELY SECURED IN PLA CONCRETE PLACEMENT. REINFORCING STEEL SHALL NOT BE FIELD BENT EXCEPT AS NOT DESIGN DRAWINGS. WELDING OF REINFORCING STEEL SHALL NOT BE PERMITTED WITHOU APPROVAL OF THE ENGINEER OF RECORD EXCEPT AS NOTED ON THE DESIGN DRAWINGS.

03200 - CONCRETE WALL REINFORCING

PLACE TWO HORIZONTAL #5 BARS AT EACH FLOOR LEVEL OR TOP OF WALL ELEVATION. P CORNER BARS TO MATCH HORIZONTAL REINFORCEMENT AT EACH WALL CORNER AND INT PROVIDE TWO VERTICAL #5 BARS AT EACH WALL CORNER AND INTERSECTION. AT ALL WAL PROVIDE TWO #5 BARS OVER, UNDER, AND AT THE SIDES OF THE OPENINGS, EXTEND THE BARS THE LAP SPLICE DISTANCE PAST THE OPENING OR EXTEND AS FAR AS POSSIBLE AND PROVIDE ONE #5 BAR BY 4'-0" LONG DIAGONALLY AT EACH CORNER OF THE WALL OPENING CONCRETE SHALL BE PLACED AND CONSOLIDATED WALLS SHALL BE REINFORCED PER SC BELOW U.N.O.:

WALL	THICKNESS

ALL THICKNESS	HORIZONTAL	VERTICAL	LOCATION
	#4 AT 14"OC	#5 AT 18"OC	CENTERLINE
	#4 AT 10"OC	#5 AT 15"OC	CENTERLINE
1	#4 AT 16"OC	#5 AT 18"OC	EACH FACE
1	#4 AT 12"OC	#5 AT 18"OC	EACH FACE

EPOXY ALL HORIZONTAL STEEL INTO EXISTING FOUNDATION WITH FOUR INCH EMBEDMEN SECTION 08100 FOR EPOXY TYPE.

05000 - STRUCTURAL STEEL

DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "AISC 360-10 SPE FOR STRUCTURAL STEEL BUILDINGS". MATERIALS SHALL BE IN ACCORDANCE WITH THE FO U.N.O.:

STRUCTURAL W SHAPE	ASTM A-992	Fy = 50 KSI
S, M, AND C SHAPES	ASTM A-36	Fy = 36 KSI
STEEL ANGLES	ASTM A-36	Fy = 36 KSI
PLATE MATERIAL	ASTM A-36	Fy = 36 KSI
STRUCTURAL PIPE	ASTM A-53 GRADE B	Fy = 35 KSI
STRUCTURAL HSS	ASTM A-500 GRADE B	Fy = 46 KSI
ANCHOR RODS	ASTM F1554	Fy = 36 KSI
WOOD CONNECTION BOLTS	ASTM A-307 GRADE A	
WELDING ELECTRODES	E7018	

ALL WELDING SHALL CONFORM TO THE AWS D1.4 "STRUCTURAL WELDING CODE". ALL WEI PERFORMED BY A WASHINGTON ASSOCIATION OF BUILDING OFFICIALS (WABO) AND AMER SOCIETY (AWS) CERTIFIED WELDERS. ALL COMPLETE PENETRATION (CP) WELDS SHALL BE ULTRASONICALLY TESTED. ALL FILLET WELDS SHALL BE VISUALLY INSPECTED RE: S1.1.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER SHALL BE HOT DIPPED G AFTER FABRICATION IN COMPLIANCE WITH ASTM A-123. ALL FIELD WELDS EXPOSED TO WE BE COATED WITH BRUSH APPLIED ZINC-RICH PAINT COMPLYING WITH ASTM A-780.

ALL STRUCTURAL STEEL TO RECEIVE ONE COAT OF PAINT (PRIME COAT). PROVIDE A MININ THICKNESS OF ONE MIL. PREPARE SURFACE TO MEET REQUIREMENTS OF SSPC-SP2. TOU ABRASIONS ARE THE RESPONSIBILITY OF THE CONTRACTOR. UNO. REFER TO ARCHITECTU DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION RELATING TO FINISH PAIL FINISH REQUIREMENTS.

				S	ΤRΙ	JCTURAL	N	ΟΤΕS						
LOCAL CODES.	06000 - WOOD FRAMI FRAMING CONNECTO AS MANUFACTURED APPROVED BY ENGIN WHERE STRAPS CON	NG NOTES DRS, ACCESSORIES, AND I BY SIMPSON STRONG-TIE IEER OF RECORD. INSTAL INECT TWO MEMBERS TO	FASTENERS A . EQUIVALENT .L ALL HARDW/ GETHER, PLAC	S NOTED IN T HARDWARE ARE PER MAN CE HALF OF T	HE PLANS MY BE US NUFACTU	S AND DETAILS SHALL BE SED WITH PRIOR RERS SPECIFICATIONS. JIRED FASTENERS INTO	F M A	06610 - SHOP FABRICATI PREMANUFACTURED ME MANUFACTURED IN ACC ANSI/TPI 1-2007 "NATION/ CONSTRUCTION". A TRU	ED METAL PLATE CO ETAL-PLATE-CONNEC ORDANCE WITH IBC AL DESIGN STANDAF SS SUBMITTAL PACH	DNNECTI CTED WC SECTIO RD FOR M (AGE SH	ED WOC DOD TRI N 2303.4 METAL-I ALL BE)D TRUS USSES § 4 TRUSS PLATE-C SUBMIT	SES HALL BE ES, AND ONNECT TED FOR	DES THE ED W APP
SECTION 01200)	EACH MEMBER. PRO REQUIREMENTS AT 1 SHALL CONFORM TO SHEET S9.0.	VIDE SOILD BLOCKING AT REATED LUMBER. TYPICA FASTENING SCHEDULE P	ALL BEARING AL NAILING NO ER IBC TABLE	POINTS. SEE T SHOWN PEI 2304.10.1 OR	SECTION R PLAN, D TO THE F	I 06100 FOR FASTENER DETAIL, OR SCHEDULE FASTENING SCHEDULE ON	N [FABRICATION PER THE F STAMP AND SEAL OF A F DESIGN FOR THE SPANS	REQUIREMENTS OF I REGISTERED STATE S, LOADS, SHAPES, B	BC 2303. OF WAS	4.2. THE HINGTO POINTS	E TRUSS	DESIGN ESSIONA	DRA' L EN S, HII
IVE MATERIAL PER S SHALL BE T THE	NAILS SHALL BE CON NAIL SIZE 8d	MMON UNLESS NOTED OT DIAMETER L 0.131"	HERWISE CON LENGTH 2 1/2"	IMON NAIL DI	IMENSION	NS ARE AS FOLLOWS:	ז [ר	TRAMING, BLOCKING PA DEFLECTION CRITERIA S TOP CHORD LOADS TOP CHORD LIVE LOAD	NELS AND ALL CONL SHALL BE AS FOLLOV	VS: 25 PS	5HOWN	ON THE	: PLANS.	IHE
S SPECIFIED BY	10d 12d 16d	0.148" 0.148" 0.162"	3" 3 1/4" 3 1/2"				1	TOP CHORD DEAD LOAD TOP CHORD GROSS WIN OVERHANGS AT) ID UPLIFT CORNERS	10 PS 44.8 PS	= SF			
RING DRAWINGS,	UNLESS NOTED OTH BE 5/8 INCH DIAMETE MORE THAN 4 FEET A LOCATED NOT MORE 3"x3"x1/4" PLATE WAS	ERWISE PER SHEARWALL R WITH 7 INCHES MINIMU APART. THERE SHALL BE A THAN 12 INCHES NOR LE SHER SHALL BE PROVIDED	SCHEDULE O M EMBEDMEN MINIMUM OF SS THAN 4 1/2 FOR ALL ANC	R PLANS. ANG T INTO CONC TWO BOLTS I INCHES FRO CHOR BOLTS (CHOR BO RETE ANI PER SILL M EACH I (COUNTE	LTS AT SILL PLATES SHALL D SHALL BE SPACED NOT PIECE WITH ONE BOLT END OF THE PIECE. A RSINK PLATE WASHERS		CORNERS OVERHANG AT E EDGES FIELD TOP CHORD NET WIND F ABOVE PRESSURES LES	DGE PRESSURE	32.0 PS 44.8 PS 32.0 PS 22.3 PS 10.0 PS	F F F F F			
DEBRIS. BACKFILL ORTED. ALL BACKFILL ECHNICAL ENGINEERING	SHALL NOT BE ALLO	WED).			(E	BOTTOM CHORD LOADS BOTTOM CHORD DEAD L	OAD	5 PS	۶F			
UTE STANDARD ACI	06100 - ROUGH FRAN SAWN LUMBER SHAL DRESSING RULES" N PERCENT MAXIMUM DRYING OF ASSEMBI WEATHER OR IN CON	IING L CONFORM TO WEST CC O. 17 LATEST EDITION. SA MOISTURE CONTENT. PR LED FRAMING TO MINIMIZE ITACT WITH CONCRETE C	DAST LUMBER S AWN LUMBER S OTECT LUMBE E WOOD SHRIN DR MASONRY S	INSPECTION I SHALL BE S4S R FROM WEA IKAGE POTEN SHALL BE PRE	BUREAU (S AND SUI ATHER AN NTIAL. AL ESERVATI	(WCLIB) "GRADING AND RFACED DRIED, 19 ID PROVIDE FURTHER .L LUMBER EXPOSED TO IVE TREATED U.N.O. PER		DEFLECTION LIMITATION LIVE LOAD DEFLECTION TOTAL LOAD DEFLECTIO		L/720 L/480				
LL BE APPROVED BY	PLAN. LUMBER SPEC U.N.O. PER PLAN/SCH	CIES, GRADE, AND PROPE HEDULE:	RTIES FOR EA	CH USE/LOCA	ATION SH	ALL BE AS FOLLOWS	E	MATERIALS. PROVIDE EA BEARING LOCATIONS, PE PERMANENT BRACING L	ACH TRUSS WITH TH ERMANENT BRACING OCATIONS & TRUSS	E STRUC G LOCAT	TURAL	BUILDIN C THE	IG COMP TRUSS D	ONEI)ESIC
LORIDE ION SHALL	USE/LOCATION WALL STUDS/BLOCKI 2X, 3X 4" WIDE	SPECIES ING HEM-FIR	Fb GRADE (PS STUD 67	9 Fv Fc 61) (PSI) (PS 75 150 405	p Fc I) (PSI) 5 800	E (PSI) 1.2E6	S I F	STORE, INSTALL & BRAC INFORMATION (BCSI) "GL PLATED-WOOD TRUSSES ALL TEMPORARY BRACIN	E TRUSSES IN ACCC JIDE TO GOOD PRAC S" & BCSI B1 THROU NG; SEE BCSI-2 FOR	ORDANCI CTICE FO GH B11 (TYPICAL	E WITH Y R HANE QUICK R	WTCA/TI)LING, IN (EFEREN)RARY E	PI (SBCA) ISTALLIN ICES. THI BRACING	BUIL G & E E CO REQI
ENT (7) LOCATION AND	2X, 3X 6" & WIDER	HEM-FIR	NO. 2 85	50 150 40	5 1300	1.3E6		THE CONTRACTOR SHAL DRAWINGS AND PLANS.	L INSTALL ALL PERN REFERENCE BCSI-B	MANENT 3 FOR T	BRACIN' /PICAL	IG AS IN' PERMAN	DICATED	
	WALL PLATES 2X4, 3X4 2X6, 3X6	HEM-FIR HEM-FIR	STUD 6 NO. 2 8	75 150 40 50 150 40	5 800 05 1300	1.2E6 1.3E6		MINIMUM BEARING FOR T DIAMETER x 3" TOE NAIL SUPPORT OF TRUSS.	TRUSSES SHALL BE ED, ONE EACH SIDE.	3 1/2". SI . AS A MI	Ecure ⁻ Nimum	TRUSSE PROVID'	:s to tof e H2.5a f	? PLA IURR
	JOISTS 2X, 3X	HEM-FIR	NO. 2 8	50 150 40	05 1300	1.3E6		06620 - STRUCTURAL GL GLUED-LAMINATED MEM	UED LAMINATED TIN IBERS SHALL HAVE A	MBER America			F TIMBER	
NOT SPECIFIED BY	2X, 3X 4X	DOUGLAS FIR-LARCH DOUGLAS FIR-LARCH	NO. 2 9 NO. 1 10	00 180 62 00 180 62	25 1350 25 1500	1.6E6 1.7E6	F	IMMEDIATELY AFTER TRI FOLLOWS:	IMMING IN EITHER SI	HOP OR	FIELD. [DESIGN	MATERIA	LPR
318-14. REINFORCING	BEAMS AND POSTS 4X 6X	DOUGLAS FIR-LARCH DOUGLAS FIR-LARCH	NO. 2 9 NO. 1 12	00 180 62 00 170 62	25 1350 25 1000	1.6E6 1.6E6		USE SIMPLE SPAN BEAM CONTINUOUS BEAM CANTILEVER BEAM	COMBINATION 24F-V4 24F-V8 24F-V8	I SYMBO	L	SPECIE DF/DF DF/DF DF/DF	<u>-</u> S (CAME STAN ZERC ZERC
IC OR FIBER MESH	06200 - PRESERVATIV PRESERVATIVE TREA SUPPORT OF THE BL APPURTENANCES TH ROOF, EAVE, OVERH THE SURFACE OR AT	VE TREATED WOOD PROD ATED WOOD SHALL BE RE JILDING, BALCONIES PORC HAT ARE EXPOSED TO THE ANG OR OTHER COVERIN ⁻ JOINTS BETWEEN MEMB	QUIRED FOR A QUIRED FOR A CHES, OR SIMII E WEATHER W G TO PREVEN ERS.	ALL WOOD TH _AR PERMAN ITHOUT ADEC T MOISTURE (IAT FORM ENT BUIL QUATE PF OR WATE	IS THE STRUCTURAL DING ROTECTION FROM A ER ACCUMULATION AT		UNEXPOSED GLUED-LAN OTHERWISE. EXPOSED (06630 - STRUCTURAL CO STRUCTURAL COMPOSIT SHALL BE THE SIZE AND	AINATED TIMBER SH. GLUED LAMINATED T OMPOSITE LUMBER (TE LUMBER SHALL C TYPE SHOWN ON TH	ALL BE IN FIMBER S (SCL) SONFORM HE DRAV	IDUSTR HALL B I TO AL VINGS A	L PERTI	ADE. TYPI ARANCE (NENT PR JFACTUR	Cal, Clas Covis Ed B
E ON SHEET S6.0. 3 AROUND CORNERS	ALL WOOD INSTALLE FOUNDATION WALL I POSTS OR COLUMNS	D ABOVE GROUND AND R LESS THAN 8 INCHES FRO S SUPPORTING PERMANEI	ESTING ON AN M EXPOSED E/ NT STRUCTUR	I EXTERIOR C ARTH. ES AND SUPF	CONCRET	E OR MASONRY BY A CONCRETE SLAB		APPROVED EQUAL. STO SPECIFICATIONS. ALL ME OF RECORD APPROVAL. ALLAOWABLE DESIGN M	RAGE, ERECTION, AI EMBERS SHALL NOT ATERIAL PROPERTIE	ND INSTA HAVE NO ES SHALI	LLATIO	N SHALI 3 OR DR FOLLOV	_ BE PER ILLED HO VS (ALL U	MAN LES JNITS
	OR FOOTING THAT IS 1. IF LOCATED I ABOVE THE S	S IN DIRECT CONTACT WIT N BASEMENTS ON A CONG LAB AND SEPARATED THE	"H THE EARTH. CRETE PIER OI EREFROM BY A	EXCEPT; R METAL PED N IMPERVIOU	ESTAL 1 JS MOIST	INCH URE BARRIER.	1	ORIENTA TIMBERSTRAND LAMINA COLUMN PLANK	TION TED STRAND LUMBE	R (LSL)	Fb 1700 1900	Fv 400 150	Fc(perp) 680 435	F 14 14
	2. IF IN AN ENCL PERIPHERY A EXPOSED GR	LOSED CRAWL SPACE OR ND SUPPORTED BY A CON OUND AND SEPARATED T	AN UNEXCAVA	ATED AREA W OR PEDESTAL	/ITHIN TH L MORE T OUS MOI	E BUILDING 'HAN 8 INCHES FROM STURE BARRIER		BEAM RIM MICROLAM LAMINATED V		/1)	2325 2325	310 310	800 800	20
	3. SLEEPERS AI MOISTURE B	ND SILLS ON A CONCRETE ARRIER SEPARATION WITI	E SLAB ON GRA H EXPOSED EA	ADE THAT DO ARTH.			F	BEAM PARALLAM PARALLEL ST COLUMN BEAM	FRAND LUMBER (PSL	_)	2600 2400 2900	NA NA 290	NA NA 750	25 25 25
	PRESERVATIVE TREA	ALLS BELOW GRADE.	/IERICAN WOO	D PRESERVE	RS' ASSC		(08100 - EPOXY ADHESIVE	E ANCHORS					
IN PLACE PRIOR TO S NOTED IN THE VITHOUT PRIOR WINGS	SPECIFICATION C2 A ALL FASTENERS (NA WITH TREATED LUME A153 OR STAINLESS	ND C9 OR APPLICABLE ST ILS, BOLTS, ANCHOR BOLT BER SHALL BE CORROSIO STEEL.	ANDARDS. TS, PLATES, HA N RESISTANT (ANGERS, ETC G-185 HOT DII	.) IN CON PPED GAI	TACT LVANIZED PER ASTM	E F	CONCRETE EPOXY SPECIFIED IN THE ANCHOR ROD, THREADE PER ESR-2508.	E DRAWINGS SHALL ED ROD, OR REINFOR	BE SIMP RCING DI	SON ST AMETEI	'RONG-T R AND E	"IE SET-XI MBEDME	P EP NT P
ION. PROVIDE AND INTERSECTION. ALL WALL OPENINGS	06300 - JOIST AND BI JOIST AND BEAM HA TIE. EQUIVALENT HA BEAM HANGERS SHA UNLESS NOTED OTH	EAM HANGERS NGERS AS NOTED IN THE RDWARE MAY BE USED W ALL BE INSTALLED PER MA IERWISE PER PLANS OR D	PLANS SHALL /ITH PRIOR API NUFACTURER DETAILS:	BE AS MANUI PROVAL BY E S' SPECIFICA	FACTURE INGINEER ITIONS AN	ED BY SIMPSON STRONG- R OF RECORD. JOIST AND ND SHALL BE AS FOLLOWS	S	08200 - EXPANSION ANC CONCRETE EXPANSION ANCHORS S STRONG-BOLT WEDGE A INSTALLATION PER SECT	HORS PECIFIED IN THE DR ANCHOR. ANCHOR D TION 4.3 OF ESR-177	AWINGS IAMETEF 1.	SHALL ₹ AND E	BE SIMF MBEDM	'SON STF ENT PER	≀ong Plai
BLE AND HOOK. PENING. ALL PER SCHEDULE	MEMBER SIZE SAWN LUMBER	HANGEF LUS OR WHERE	R HUS SERIES T NOT NOTED S	O MATCH LUI PECIFICALLY	MBER SIZ BELOW	Έ		08300 - SCREW ANCHOR CONCRETE SCREW ANCHORS SPEC ANCHOR DIAMETER AND	S IFIED IN THE DRAWI DEMBEDMENT PER F	NGS SHA PLAN. INS	ALL BE S STALLA	Simpson Tion Pe	N STRON(R ESR-27	G-TIE 13.
EDMENT. RE: NOTES	GLUED LAMINATED E 3 1/8" LGU3.25-SDS 3 1/2" HGU3.63-SDS 5 1/8" HGU5.25-SDS 5 1/4" HHGU5.50-SE 5 1/2" HHGU5.62-SE 6 3/4" HHGU7.00-SE 8 3/4" HHGU9.00-SE	BEAMS (H = BEAM DEPTH W/(16) SDS 1/4x2 1/2" FAC W/(36) SDS 1/4x2 1/2" FAC W/(36) SDS 1/4x2 1/2" FAC W/(44) SDS 1/4x2 1/2" FAC SW/(44) SDS 1/4x2 1/2" FAC SM(44) SDS 1/4x2 1/2"	TYPICAL) (DF (E, (12) SDS 1/4 XE, (24) SDS 1/4 XE, (24) SDS 1/4 ACE, (28) SDS 1 ACE, (28) SDS 1 ACE, (28) SDS 1 ACE, (28) SDS 1	CAPACITY / HI x2 1/2" JOIST ix2 1/2" JOIST ix2 1/2" JOIST /4x2 1/2" JOIS /4x2 1/2" JOIS /4x2 1/2" JOIS /4x2 1/2" JOIS	F CAPACI (6720) (14145 (14145 (14145 T (17845 T (17845 T (17845 T (17845	TY) / 4840) 5 / 10185) 5 / 10185) 5 / 12850) 5 / 12850) 5 / 12850) 5 / 12850)								
DANCE WITH THE 10 SPECIFICATION	10 3/4" HHGU11.00-S LSL 1 1/2" x 11 7/8" (2) 1 3/4" x 11 7/8"	DS W/(44) SDS 1/4x2 1/2" F MIU1.56/11 W/(2 HHUS410 W/(30)	ACE, (28) SDS 0) 16d FACE, (2) 16d FACE, (10	1/4x2 1/2" JOI () 10d x 1 1/2" 、) 16d JOIST	st (18480 Joist	(2880) (5635)								
THE FOLLOWING	3 1/2" x 11 7/8" PROVIDE HUC HANG	HHUS410 W/(30) ER FOR BEAM SIZE SPECI) 16d JOIST IFIED FOR END	OF BEAM CC	ONDITION	(5635) S.								
	SHRINKAGE IN WOOI ASSEMBLIES OF WOO EXTERIOR FINISHES SHRINKAGE. THE US MEMBERS PRIOR TO	D FRAMING IS DUE TO LOS OD COMPONENTS. PLUMI SHALL BE DESIGNED AND E OF KILN DRIED LUMBER APPLICATION OF FINISHE	SS OF MOISTUI BING, ELECTRI BUILT TO ACC AND PROVIDIN S WILL HELP C	RE CONTENT CAL, AND ME COMMODATE NG A DRYING CONTROL BUT	AND TO CHANICA 1/4 INCH PROCES T WILL NC	Compression of L Systems as well as Per Floor Wood S to the Framing Dt Eliminate Shrinkage.								
ALL WELDING SHALL BE AMERICAN WELDING HALL BE 61.1.	06500 - WOOD SHEAT STRUCTURAL WOOD PLYWOOD ASSOCIAT ORIENTED STRAND E THICKNESS, SPAN RA	THING O SHEATHING PANELS SHA FION. WOOD SHEATHING F BOARD (OSB) PANELS SHA ATING, AND FASTENING U	ALL HAVE APA PANELS SHALL ALL BE EXPOSI NLESS NOTED FDGF	GRADE TRAD BE C-D INT A JRE 1. PANEL OTHERWISE FIFI D	EMARK C IPA WITH IS SHALL PER PLA	OF THE AMERICAN EXTERIOR GLUE (CDX). HAVE THE FOLLOWING N:								
PPED GALVANIZED TO WEATHER SHALL	ROOF: 5/8' FLOOR: 3/4' SHEARWALL: 7/10	" 40/20 C-D APA CDX " 48/24 C-D T&G 6" C-D EXTERIOR GLUE	NAILS 8d AT 6 10d AT SEE SC	NAILS NAILS 8d AT 12 6" 10d AT 1 CHEDULE SHF	." 2" EET S1.1									
A MINIMUM FRY-FILM 2. TOUCHUPS OF IITECTURAI	EXTERIOR WALL: 7/10 ALL ROOF SHEATHIN	6" D-D EXTERIOR GLUE	10d AT	6" 10d AT 1	2" NDICULAI	R TO SUPPORTS AND IN								
SH PAINT OR OTHER	A STAGGERED PATT ROOF SHEATHING JO SHEATHING SHALL B	ERN UNLESS NOTED OTH DINTS SHALL NOT BE REQ E BLOCKED AT ALL EDGE	ERWISE PER P UIRED UNLESS S WITH 2X OR	PLAN. BLOCKI S NOTED OTH 3X FRAMING	NG AT IN IERWISE PER SHE	TERMEDIATE FLOOR AND PER PLAN. SHEARWALL ARWALL SCHEDULE.								

	5		STRUCTURAL DRAWING LIST					
BE DESIGNED AND ND THE TRUSS PLATE INSTITUTE CTED WOOD TRUSS OR APPROVAL PRIOR TO GN DRAWINGS SHALL BEAR THE NAL ENGINEER. ONS, HIPS AND VALLEYS, OVER- IS. THE DESIGN LOADS AND		SHEET S1.0 S1.1 S1.2 S2.0 S2.1 S2.2 S6.0 S8.0 S8.1 S8.2 S8.3 S9.0 S9.1 S9.2 S10.0	DESCRIPTION Structural Notes Shearwall Schedule and Details Holddown Schedule and Details Basement Level Walls Over Foundation Main Frmg Over Basement LvI Shear Walls Roof Framing Over Main Level Shear Walls Typical Concrete Details Moment Frame Details Moment Frame Details Moment Frame Details Moment Frame Details Typical Wood Framing Details Typical Wood Framing Details Typical Wood Framing Details Typical Components	Rev	Rev Date 06-10-2022 06-10-2022 06-10-2022 06-10-2022 06-10-2022 06-10-2022 06-10-2022		CT ENGINEERING ING.	Structural Engineers 180 Nickerson Street Suite 302 Seattle, WA 98109 206.285.4512 (V) 206.285.0618 (F) www.ctengineering.com
ING PANELS AND REQUIRED MPONENT (SBCA) TAGS FOR S DESIGNER SHALL SPECIFY ALL ESIGN DRAWINGS. CA) BUILDING COMPONENT SAFETY LING & BRACING OF METAL- THE CONTRACTOR SHALL INSTALL IG REQUIREMENTS.								03/25/22
ED ON THE TRUSS DESIGN RACING REQUIREMENTS U.N.O. FOP PLATE WITH (2) 0.148" A HURRICANE CLIP AT EACH ER CONSTRUCTION (AITC) F END SEALER APPLIED RIAL PROPERTIES SHALL BE AS						DATE		
CAMBER STANDARD ZERO ZERO YPICAL, UNLESS NOTED E CLASS PER ARCHITECT.						REVISION		
PROVISIONS OF ASTM D5456 AND JRED BY ILEVEL TRUS JOIST OR ER MANUFACTURER HOLES WITHOUT PRIOR ENGINEER L UNITS ARE IN PSI): rrp) FC E						No.	1-0" TES:	022
1400 1.3E6 1400 1.3E6 2050 1.55E6 2050 1.55E6 2500 1.9E6 2500 1.8E6 2900 2.0E6						JOB #: 21162	ENG: BJM CAD: JMA SCALE: 3/4" = KEY ISSUE DA	SD: SD DD: DD CD: CD CD: CD PERMIT: 03.25.2 OTHER: BD
T-XP EPOXY ADHESIVE. MENT PER PLAN. INSTALLATION								
STRONG-TIE ER PLAN. DNG-TIE TITEN HD. -2713								
							otes FI,	A 98040
							DIPLER REMODE	8429 SE 33RD PLACE MERCER ISLAND, W
							S1.	.0

			SHEARW	ALL SCHE	DULE - 7/16" APA	A RATED SHEATH	ING W/ HEM-FIR STU	JDS AND HEM-FIR PL	ATES		
WALL	WALL PANEL		FIELD	BOTTO		RIM OR BLOCK	ING TO TOP PLATE	CONN. (10)	FRAMING	FOUNDATION	ANCHOR BOLT
TYPE	SHEATHING	EDGE NAILING 3	NAILING	ROWS	SPACING	0.148"x3.25" TOENAIL	LTP4 DIRECT TO FRAMING	A35 ONLY	AT ADJOINING PANEL EDGES (5)	SILL PLATE	SPACING 5/8" DIA. 7" EMBED (13)
P6TN	7/16" SHT. ONE SIDE	6" O.C.	12" O.C.	(1)	4" O.C.	4" O.C.	N/A	N/A	2x	2x	48" O.C.
P6	7/16" SHT. ONE SIDE	6" O.C.	12" O.C.	(1)	4" O.C.	N/A	24" O.C.	16" O.C.	2x	2x	48" O.C.
P4	7/16" SHT. ONE SIDE	4" O.C.	12" O.C.	(2)	6" O.C.	N/A	16" O.C.	12" O.C.	(2)2x OR 3x	2x	32" O.C.
P3	7/16" SHT. ONE SIDE	3" O.C.	12" O.C.	(2)	4" O.C.	N/A	12" O.C.	10" O.C.	(2)2x OR 3x	2x	24" O.C.
P2	7/16" SHT. ONE SIDE	2" O.C.	12" O.C.	(3)	6" O.C.	N/A	10" O.C.	10" O.C.	(2)2x OR 3x	2x	18" O.C.
2P4	7/16" SHT. BOTH SIDES	4" O.C.	12" O.C.	(3)	5" O.C.	N/A	10" O.C.	10" O.C.	(2)2x OR 3x	2x	16" O.C.
2P3	7/16" SHT. BOTH SIDES	3" O.C.	12" O.C.	(3)	4" O.C.	N/A	8" O.C.	8" O.C.	(2)2x OR 3x	2x	12" O.C.
2P2	7/16" SHT. BOTH SIDES	2" O.C.	12" O.C.	(3)	3" O.C.	N/A	6" O.C.	6" O.C.	(2)2x OR 3x	2x	8" O.C.

(#) REFERS TO KEYNOTES IN DETAIL 19 THIS SHEET

SHEARWALL SCHEDULE NOTES :

11

(4)

MEMBER.

1. STUDS SHALL NOT BE SPACED MORE THAN 16" O.C. 2. RE: S1.0 SECTION 06100 "ROUGH FRAMING" FOR REQUIRED WALL STUD AND PLATE SPECIES AND GRADE. 3. RE: S1.0 SECTION 06160 "WOOD SHEATHING" FOR REQUIRED SHEAR WALL SHEATHING, THICKNESS AND GRADE. ALL SHEAR WALL

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4. SHEATHING PANELS MAY BE INSTALLED EITHER HORIZONTALLY OR VERTICALLY WITH ALL PANEL EDGES BACKED/BLOCKED WITH

5. FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS SHALL NOT BE LESS THAN 3" NOMINAL AND NAILS SHALL 6. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6" O.C. ON EITHER SIDE, PANEL

JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3" NOMINAL OR THICKER AND NAILS

7. NAILS FOR PLYWOOD AND OSB PANEL EDGE AND FIELD NAILING SHALL BE 8D COMMON (0.131" X 2 1/2"). 9. FLOOR DIAPHRAGM NAILING SHALL BE PLACED BETWEEN THE SPACING CALLED OUT FOR BOTTOM PLATE NAILING. DO NOT OVER

10. ANCHOR BOLTS SHALL BE GALVANIZED 5/8" DIAMETER A-307 AND SHALL BE SECURED IN PLACE PRIOR TO CONCRETE POUR.

11. GALVANIZED 3" X 3" X 0.229" (MIN.) PLATE WASHERS ARE REQUIRED AT EACH ANCHOR BOLT - SEE 8 THIS SHEET FOR

12. LTP4 FRAMING PLATES SHALL BE INSTALLED WITH 12-8D X 1 1/2" (0.131" X 2 1/2") NAILS. RE: DETAILS 1, 2, 3 & 6/S1.1. 13. A35 FRAMING ANGLES SHALL BE INSTALLED WITH 12-8D X 1 1/2" (0.I31" X 1 1/2") NAILS. RE: DETAILS 1, 2 & 3/S1.1.

14. ALL NAILS INTO PRESSURE TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED CONFORMING TO ASTM 153 OR STAINLESS

16. WHERE BOTTOM PLATE NAILING SPECIFIES A SPACING OF 4 INCHES OR LESS NAILS SHALL BE INSTALLED IN TWO ROWS OFFSET 17. GALVANIZED EXPANSION ANCHORS OF SIMILAR. DIAMETER AND EMBEDMENT ALLOWED AT INTERIOR BEARING AND PARTY

18. 2-2X'S IN LIEU OF 3X'S AT PANEL EDGES ACCEPTABLE PROVIDED STUDS ARE ATTACHED PER 10/S1.2 SIM. AND BOTTOM PLATE

19. WHERE BUILDING OFFICIALS ALLOW, OSB SHEATHING MAY BE APPLIED OVER 1/2" OR 3/8" GYPSUM WALL BOARD PROVIDED

ALL PANEL EDGES SHALL BE FASTENED TO STUDS OR BLOCKING. (3) <u>PANEL EDGE NAILING:</u> NAILING AT ALL OUTER EDGES OF SHEATHING PANELS IN SHEARWALLS SHALL BE FASTENED PER THE SHEARWALL SCHEDULE. (4) FIELD NAILING (4) WITHIN THE FIELD OF THE PANEL, AT FRAMING MEMBERS, THE PANELS ARE LESS CLOSELY FASTENED. (5) <u>FRAMING AT ADJOINING PANEL EDGES:</u> WHERE TWO PIECES OF PLYWOOD JOIN ON A FRAMING MEMBER, THE PANEL EDGE NAILING FROM EACH PANEL IS TO BE STAGGERED. SOME WALLS REQUIRE 3 INCH NOMINAL FRAMING MEMBER (EITHER A STUD OR BLOCKING) AT ADJOINING PANEL EDGES (SEE SHEARWALL SCHEDULE FOR WALL TYPES REQUIRING 3 INCH NOMINAL FRAMING MEMBERS AT ADJOINING PANEL EDGES). DOUBLED STUDS ARE GENERALLY NOT ACCEPTABLE FOR THIS APPLICATION. WHERE A SINGLE PANEL EDGE LANDS ON A FRAMING MEMBER, A 2 INCH NOMINAL FRAMING MEMBER SHALL BE ACCEPTABLE (AT ENDS OF WALLS FOR EXAMPLE). BLOCK ALL PLYWOOD EDGES NOT SUPPORTED BY FRAMING MEMBERS AND NAIL W/PANEL EDGE NAILING. (6) (6) <u>BOTTOM PLATE:</u> (7) BOTTOM PLATE NAILING: LOCATE THE NAILING THROUGH THE BOTTOM PLATE SO AS TO FULLY PENETRATE THE SOLID BLOCKING OR CONTINUOUS RIM BENEATH THE FLOOR SHEATHING, SPACED AS PER THE SHEARWALL SCHEDULE. (8) ROOF DIAPHRAGM BOUNDAY EDGE NAILING: FLOOR DIAPHRAGM NAILING SHALL BE INSTALLED BETWEEN THE SPACING SHOWN FOR BOTTOM PLATE NAILING. LOCATE ADJOINING PANEL EDGES OF FLOOR SHEATHING AWAY FROM SHEARWALLS. FIELD NAILING OF FLOOR SHEATHING MAY BE OMITTED AT SHEARWALL BOTTOM PLATE NAILING. RE: NOTES 06500 (9) (9) TRUSS BLOCKING PANEL JOIN ADJACENT TRUSS BLOCKING PANEL WITH FACE NAILING AS SPECIFIED ABOVE SHIM WITH FULL HEIGHT SHIMS, ADJUST FACE NAIL LENGTHS. REFER TO PLANS FOR ADDITIONAL SEISMIC CONNECTIONS AT THE FLOOR OR ROOF LEVEL. (10) TRUSS BLOCKING PANEL TO TOP PLATE CONNECTION: THE CONTINUOUS TRUSS BLOCKING PANEL THAT IS PART OF THE SHEARWALL (10) ASSEMBLY SHALL BE CONNECTED TO THE DOUBLE TOP PLATE OR FOUNDATION SILL PLATE WITH APPROVED CONNECTORS AND SPACED PER THE SHEARWALL SCHEDULE. (11) DOUBLE TOP PLATE: LAP AND SPLICE - SEE PLANS FOR ADDITIONAL SEISMIC CONNECTIONS AT THE FLOOR OR ROOF LEVEL. (12) FOUNDATION SILL PLATE ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE TREATED. THE FOUNDATION SILL PLATE SHALL BE EITHER 2 INCH NOMINAL OR 3 INCH NOMINAL DEPENDING ON THE SHEARWALL SCHEDULE. (13) ANCHOR BOLTS FULL DIAMETER ANCHOR BOLTS, ASTM A-307 SHALL BE SECURED IN PLACE PRIOR TO PLACING CONCRETE. MINIMUM EMBEDMENT IS 7 INCHES. MIN. (2) BOLTS PER PIECE OF PLATE, W/(1) BOLT NOT MORE THAN 12" FROM END OF PIECE. (14) PLATE WASHERS: (14) (12) PLATE WASHERS SHALL BE REQUIRED FOR FOUNDATION SILL PLATE CONNECTIONS, 3" X 3" X 1/4" MINIMUM. DO NOT RECESS BOLTS IN SILL PLATE UNLESS SPECIFICALLY DETAILED ELSEWHERE. (15) SQUASH BLOCKS:

(1) <u>SHEARWALL TYPE W1</u> SHEATHING: 7/16" CD-CC SHEATHING

NAILING

STUD SPACING:

ROOF THICKNESS:

BOTTOM PLATE NAILING

APPROVED RIM PRODUCTS:

CONDITIONS DESCRIBED.

(2) WALL SHEATHING:

(CLOSEST SPACING)

ANCHOR BOLT:

RIM/BLOCKING:

APPLIED DIRECTLY TO FRAMING

16"O.C. MAX.

0.148" DIA. NAILS AT 4" O.C./SG=0.50

NO. PIECES/THICKNESS

5/8" DIA., 7" EMBED.

TRUS JOIST ER-4979 TIMBERSTRAND LSL 2.0E, PARALLAM PSL 2.0E TJ-STRAND, MICROLAM LVL RIM BOARD

SUBSTITUTIONS TO ABOVE REQUIRE ENGINEER OF RECORD APPROVAL PRIOR TO INSTALLATION. SUBMIT DOCUMENTATION BY A CODE APPROVED AGENCY. CONFIRMING THE REQUIRED CAPACITIES AND MINIMUM NAIL SPACING FOR THE

SHEATHING PANELS MAY BE INSTALLED EITHER VERTICALLY OR HORIZONTALLY.

USE LENGTH DIA.

BOTTOM PLATE/FRAMING 3 1/4" X 0.148"

SPECIAL INSPECTION: PER JURISDICTION

STUDS AND PLATE: HEM-FIR #2 OR BETTER

VERTICAL LOAD TRANSFER CAPACITY3300 LB./FT.

RIM, BLOCKING

(1) ROWS 0.148" DIA. AT 4" O.C.(1) / 1.25"

(2) ROWS 0.148" DIA. AT 4" O.C.(1) / 1.75"

(3) ROWS 0.148" DIA. AT 4" O.C.(1) / 3.50"

LATERAL LOAD TRANSFER CAPACITY (1.25") 600 LB./FT.

LATERAL LOAD TRANSFER CAPACITY (3.50") 1200 LB./FT.

PANEL EDGE NAILING 2 3/8" X 0.148"

FLOOR THICKNESS: 23/32"

IN THE FLOOR CAVITY OF PLATFORM FRAMING POST LOADS SHALL BE PROVIDED WITH ADDITIONAL STIFFENERS EQUAL TO THE POST SIZE FROM ABOVE THAT CONTINUES THROUGH THE FLOOR. (16) <u>DIAPHRAGN</u> SEE (1) FOR SHEARWALL, FLOOR AND ROOF DIAPHRAGM THICKNESS.

- (17) CONCRETE BASE (17) CONCRETE FOUNDATION OR BASE.
- (18) (18) <u>HOLDOWN</u>

SEE SHEET S1.2 FOR HOLDOWN DETAILS AND ADDITIONAL STUDS REQUIRED.

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CT ENGENERING Inc. Inc. 180 Nickerson Street Suite 302 Seattle, WA 98109 206.285.4512 (V) 206.285.0618 (F) www.ctengineering.com
03/25/22
DATE
REVISION
JOB #: 21162 ENG: BJM CAD: JMA CAD: JMA SCALE: 3/4" = 1'-0" KEY ISSUE DATES: SD: SD DD: DD CD: CD CD: CD PERMIT: 03.25.2022 OTHER: BD
Shearwall Schedule and Details PIPER REMODEL 8429 SE 33RD PLACE MERCER ISLAND, WA 98040
S1.1

4/2022 4:16:47 PM 21162 Piper Residence Addition\STRUCT/21162 Piper Residence Addition (R19).vt

FOUNDATION LEGEND

FOUNDATION SCHEDULE

MARK	DEPTH	WIDTH	LENGTH	REINFORCING	DETAILS
(F1)	8"	1'-4"	CONT.	(2) #4 CONT.	FTG. W/ STEM WALL: 6&7/S6.0
FS	12"	1'-6"	CONT.	(2) #4 CONT.	TYP. THICKENED SLAB FOOTING
(F24)	12"	24"	24"	(2) #4 EA. WAY	POST FTG.: 9/S6.0 16&17/S6.0
F30	12"	30"	30"	(3) #4 EA. WAY	POST FTG.: 9/S6.0 16&17/S6.0
F36	12"	36"	36"	(3) #4 EA. WAY	POST FTG.: 9/S6.0 16&17/S6.0
(F1)	8"	1'-4"	CONT.	(2) #4 CONT.	TURNED DOWN SLAB EDGE 6,7,8/S6.0

FOUNDATION NOTES

- 1. ALL SOIL BEARING SURFACES ARE SUBJECT TO INSPECTION AND APPROVAL BY THE
- GEOTECHNICAL ENGINEER PRIOR TO REINFORCING AND CONCRETE PLACEMENT. 2. CENTER INTERIOR FOOTINGS ON WALLS OR COLUMNS TYPICAL U.N.O.
- 3. VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION.
- 4. SEE ARCHITECTURAL PLANS FOR WALL LOCATIONS. 5. CONCRETE WALLS ARE 8" THICK TYPICAL U.N.O.
- 6. SEE SHEET S2.1 FOR WOOD FRAMING LEGEND, NOTES, AND SCHEDULES.
- 7. PROVIDE 4" DIAMETER PERFERATED FOOTING DRAINS AT PERIMETER OF FOUNDATIONS TYPICAL. PROVIDE 4" DIAMETER TIGHTLINES FOR DOWNSPOUTS, EXTEND TO DAYLIGHT.

21.1 21.2 (21.3) (21.4) (21.5)

NOTE:

PLANS PREPARED USING ARCHITECTURAL BACKGROUNDS RECEIVED 02/28/2022.

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HD 11

PLANS PREPARED USING ARCHITECTURAL BACKGROUNDS RECEIVED 02/28/2022.

99825498 1162 Piper

Simpson Strong—Tie[®] Strong Frame[®] and the Yield—Link[™] structural fuse are protected under one or more of the following US patents and applications: US patent No. 8,001,734 B2, US patent No. 8,375,652 B2, and US patent publication No. 2015/0159362, and must be supplied or licensed through Simpson Strong-tie.

GENERAL NOTES:

- SUPPLIED OR LICENSED THROUGH SIMPSON STRONG-TIE.
- REGISTERED COMPANY.
- 2018, 2015 AND 2012 INTERNATIONAL BUILDING CODE
- AISC SEISMIC PROVISIONS (ANSI/AISC 341-05, 341-10, 341-16)

- DESIGNER.
- CHORDS AND COLLECTORS AND FOUNDATIONS

- AND FRAME STABILITY / OVERTURNING.
- 12. DESIGNER MUST DETAIL ACTUAL FOOTING / GRADE BEAM SIZE AND REINFORCING.
- PRIOR TO CONSTRUCTION.
- SUCH CHANGES.
- 17. ALL HARDWARE CALLED OUT IS SIMPSON STRONG-TIE[®].

MATERIAL

- 2. W-SECTIONS (HOT ROLLED SECTIONS): ASTM A992

- 6. BEAM TOP FLANGE WOOD NAILER BOLT: ASTM A307 GR. A 7. CARRIAGE BOLTS: ASTM A307 GR. A
- 9. GROUT: ASTM C1107, MINIMUM 5,000 PSI COMPRESSIVE STRENGTH

INSTALLATION AND FIELD MODIFICATIONS COMPANY INC. PRODUCTS.

- BULLETINS, FAQS, AND OTHER PERTINENT INFORMATION.

- THE FRAME. SOME PRE-INSTALLED ITEMS MAY NOT USE ALL HOLES.
- 8. REFER TO DETAIL 11/SMF3 FOR CONNECTION PROTECTED ZONE.

INSPECTIONS

- BY THE DESIGNER.
- REQUIREMENTS.

MODEL NO.	ROD SIZE & NUMBER	LENGTH (in)
MFSL-14-6	$4 - \frac{3}{4}$	14
MFSL-14-HS6	$4 - \frac{3}{4}$	14
MFSL-18-6	$4 - \frac{3}{4}$	18
MFSL-18-HS6	$4 - \frac{3}{4}$	18
MFSL-24-6	$4 - \frac{3}{4}$	24
MFSL-24-HS6	$4 - \frac{3}{4}$	24
MFSL-30-6	$4 - \frac{3}{4}$	30
MFSL-30-HS6	$4 - \frac{3}{4}$	30
MFSL-36-6	$4 - \frac{3}{4}$	36
MFSL-36-HS6	$4 - \frac{3}{4}$	36

THE MFSL ANCHOR ASSEMBLIES HAVE BEEN ENGINEERED TO PROVIDE A COMPLETE ANCHORAGE SOLUTION MEETING THE 2012 AND 2015, 2018 INTERNATIONAL BUILDING CODE REQUIREMENTS FOR BOTH TENSION AND SHEAR.

ANCHOR RODS AND THE MFTPL TEMPLATE ARE INCLUDED PRE-ATTACHED WITH THE ASSEMBLY.

INSPECTION IS EASY; THE HEAD IS STAMPED WITH A "NO EQUAL" SYMBOL FOR IDENTIFICATION, BOLT LENGTH, BOLT DIAMETER, AND OPTIONAL "HS" FOR HIGH STRENGTH IF SPECIFIED.

4

COLLECTOR DETAILS

A. LINK FLANGE AND LINK STEM

4:16:55 PM Piper Residence Addition\STRUCT\21162 Piper Residence Addition (R1

FRAMING				S9.0
				PI P
VALL : D SCHEDULE				ypic PER 29 SE (
<u>[]</u>				Cal V REN 33RD 1 1 ISLA
	FOR COMPLETE NAILING SCHEDULE.			V OO(10DE PLACE ND, W
EARWALL SCHEDULE CKING REQUIREMENTS TRANSFER BEAM	 a. COMMON OR BOX NAILS ARE PERMITTE b. FASTENING SCHEDULE BASED ON IBC 1 REQUIRED WHEN SPECIFIED ELSEWILLE 	STRUCTURAL NOTES ED TO BE USED EXCEPT WHERE NOTE FABLE 2304.10.1 AND PROVIDES THE N FRE IN THESE PLANS PROVIDE NATION	ED OTHERWISE. MINIMUM NAILING G AS SPECIFIED, SEE IBC	1 F1 1 A 980.
O SCHEDULE	 30. BRIDGING OR BLOCKING TO JOIST, RAFTER OR TRUSS 31. WOOD STRUCTURAL PANELS TO ERAMING SUBEL COD TO STRUCTURAL 	(2) 8d COMMON (2 1/2" X 0.131"); OR (2) 3" X 0.131" NAILS SEE SHEARWALL SCHEDULE SEE	EACH END, TOENAIL SECTION06160	-am
	29. JOIST TO BAND JOIST OR RIM JOIST	 (2) 16d COMMON (3) 3" X 0.131" NAILS (3) 16d COMMON (3 1/2" X 0.162"); OR (4) 3" X 0.131" NAILS 	FACE NAIL	ing
OD RFAM	28. LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	(2) 20d COMMON (4" X 0.192"); OR (3) 3" X 0.131" NAILS (3) 16d COMMON (3 1/2" X 0.162"); OR (4) 3" X 0.131" NAILS	END JOIST OR RAFTER, FACE NAIL FACE NAIL	Detai
		3" X 0.131" NAILS	OPPOSITE SIDES 24"OC, FACE NAIL AT TOP AND BOTTOM STAGGERED ON	$\overline{\mathbf{N}}$
-	(PLANK NAD BEAM-FLOOR AND ROOF) 27. BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS	20d COMMON (4" X 0.192")	NAIL 32"OC, FACE NAIL AT TOP AND BOTTOM STAGGERED ON	
	25. 2" SUBFLOOR TO JOIST OR GIRDER 26. 2" PLANKS	(2) 16d COMMON (3 1/2" X 0.162") (2) 16d COMMON (3 1/2" X 0.162")	FACE NAIL EACH BEARING, FACE	
	TO TOP PLATE, SILL OR OTHER 24. 1" X 6" SUBFLOOR OR LESS TO EACH	3" X 0.131" NAILS (2) 8d COMMON (2 1/2" X 0.131")	FACE NAIL	
	22. JOIST TO SILL, TOP PLATE, OR GIRDER 23. RIM JOIST, BAND JOIST, OR BLOCKING	(3) 8d COMMON (2 1/2" X 0.131"); OR 3" X 0.131" NAILS 8d COMMON (2 1/2" X 0 131"): OR	TOENAIL 6"OC. TOENAII	
	21. 1" X 8" AND WIDER SHEATHING TO EACH BEARING FLOOR	(3) 8d COMMON (2 1/2" X 0.131")	FACE NAIL	
	20. 1" X 6" SHEATHING TO EACH BEARING	(2) 3" X 0.131" NAILS (2) 8d COMMON (2 1/2" X 0.131")	FACE NAIL	JOB #: JOB #: ENG: CAD: SCALE: SCALE: JD: JD: JTHER: BI OC CC
	 10P PLATES, LAPS AT CORNERS AND INTERSECTIONS 1" BRACE TO EACH STUD AND PLATE 	 (2) 16d COMMON (3 1/2" X 0.162"); OR (3) 3" X 0.131" NAILS (2) 8d COMMON (2 1/2" X 0.131"); OR 	FACE NAIL	21162 3JM MA 3/4" = 1'-0" E DATES 0 0 0,25.2022
	17. TOP OT BOTTOM PLATE TO STUD	(2) 16d COMMON (3 1/2" X 0.162"); OR (3) 3" X 0.131" NAILS	END NAIL	
	STUD TO TOP OR BOTTOM PLATE	(2) 16d COMMON (3 1/2" X 0.162"); OR	END NAIL	io z
	WALLO PANELS 16. STUD TO TOP OR BOTTOM PLATE	 (4) 3" X 0.131" NAILS (4) 8d COMMON (2 1/2" X 0.131"); OR (3) 3" X 0.131" NAILS 	TOENAIL	
	 DAIND JOIST OK BLOCKING (NOT AT BRACED WALL PANELS) 15. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING AT BRACED 	(2) 16d COMMON (3 1/2" X 0.162"); OR	12 OC FACE NAIL	RE
	14. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT	(12) 3" X 0.131" NAILS 16d COMMON (3 1/2" X 0.162"); OR 3" X 0.131" NAILS	(MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT) 16"OC FACE NAIL 12"OC FACE NAIL	NOISIN
SYSTEM FRAMING	 12. TOP PLATE TO TOP PLATE 13. TOP PLATE TO TOP PLATE, AT END JOINTS 	16d COMMON (3 1/2" X 0.162") OR 3" X 0.131" NAILS (8) 16d COMMON (3 1/2" X 0.162") OR	16"OC FACE NAIL 12"OC FACE NAIL EACH SIDE OF END	
	 10. BUILT-UP HEADER (2" TO 2" HEADER) 11. CONTINUOUS HEADER TO STUD 	16d COMMON (3 1/2" X 0.162") (4) 8d COMMON (2 1/2" X 0.131")	16"OC EACH EDGE, FACE NAIL TOENAIL	DATE
JOUS PANEL EDGES. PORTS AND AT	9. STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d COMMON (3 1/2" X 0.162");OR 3" X 0.131" NAILS	16"OC FACE NAIL 12"OC FACE NAIL	
	WALL 8. STUD TO STUD (NOT AT BRACED WALL PANELS)	16d COMMON (3 1/2" X 0.162"); 3" X 0.131" NAILS	24"OC FACE NAIL 16"OC FACE NAIL	
	 ROOF RAFTERS TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2-INCH RIDGE BEAM 	(2) 16d COMMON (3 1/2" X 0.162");OR (3) 3" X 0.131" NAILS	END NAIL	F RODO
G DFTAII	6. RAFTER OR ROOF TRUSS TO TOP PLATE (SEE SECTION 2308.7.5, TABLE	(3) 10d COMMON (3" X 0.148");OR (4) 3" X 0.131" NAILS	TOENAIL	25122
	5. COLLAR TIE TO RAFTER	(3) 10d COMMON (3" X 0.148"); OR (4) 3" X 0.131" NAILS	FACE NAIL	
	 PARALLEL RAFTER, LAPS OVER PARTITIONS (NO THRUST) (SEE SECTION 2308.7.3.1, TABLE 2308.7.3.1) 4. CEILING JOIST ATTACHED TO PARALLEL PAETER (HEEL JOINT) (SEE 	(4) 3" X 0.131" NAILS PER TABLE 2308.7.3.1	FACE NAIL	
	 CEILING JOIST NOT ATTACHED TO CEILING JOIST NOT ATTACHED TO 	(3) 3" X 0.131" NAILS (3) 16d COMMON (3 1/2" X 0.162"); OR	FACE NAIL	NG uctura Vickerson { 285.4512 (/
	FLAT BLOCKING TO TRUSS AND WEB FILLER 2. CEILING JOISTS TO TOP PLATE	16d COMMON (3 1/2" X 0.161") AT 6"OC (3) 8d COMMON (3 1/2" X 0.131"): OR	FACE NAIL	al Engi Street Suit ring.com
	BLOCKING BETWEEN RAFTERS OR TRUSSES NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	(2) 16d COMMON (3 1/2" X 0.162") (3) 3" X 0.131" NAILS	EACH END	neers 1.285.0618 (F
	BLOCKING BETWEEN RAFTERS OR TRUSSES NOT AT THE WALL TOP PLATE. TO RAFTER OR TRUSS	(3) 3" X 0.131" NAILS (2) 8d COMMON (2 1/2" X 0.131") (2) 3" X 0.131" NAILS	EACH END, TOENAIL	ING INC. wA 98109
	ROOF 1. BLOCKING BETWEEN CEILING	(3) 8d COMMON (2 1/2" X 0.131"); OR	EACH END, TOENAIL	
	IBC 2015 TABLE 2	304.10.1 FASTENING SCHED	ULE	

4:16:56 PM
 Piper Residence Addition/STRUCT/21162 Piper Residence Addition

 2x8 FLAT VALLEY ATTACH TO TRUSSES W/(3) 10d PRE-FABRICATED ROOF TRUSS @ 24"OC (TYP.) 2x8 RIDGE BEAM (ATTACH TO 2x8 VALLEY MEMBERS W/(4) 8d TN) PROVIDE BLOCKED 2'x4' OPNG FOR ATTIC VENTILATION. ROOF SHEATHING TH NG. 24"OC @ EACH END SPAN < 6'-0" 	CT ENGINEERING BNGINEERING INC. Inc. Inc. Structural Engineering.com 206.285.0618 (F) www.ctengineering.com www.ctengineering.com	
PAN > 6'-0").		
	REVISION	
	JOB #: 21162 JOB #: 21162 ENG: Designer ENG: Designer CAD: Author CAD: Author SCALE: As indicated KEY ISSUE DATES: Scale SD: SD DD: DD DD: CD DTHER: BD	
	Typical Wood Framing Details PIPER REMODEL 8429 SE 33RD PLACE MERCER ISLAND, WA 98040	
	S9.2	

THE NW 1/4 OF THE SW 1/4 OF SECTION 7, TOWNSHIP 24 NORTH, RANGE 5 EAST, W.M.

LEGAL DESCRIPTION

PER WARRANTY DEED, KING COUNTY RECORDING NO. LOT 25 OF PARKRIDGE, AS PER PLAT RECORDED IN SITUATE IN THE CITY OF MERCER ISLAND, COUNTY (

HORIZONTAL DATUM

WASHINGTON STATE PLANE COORDINATE SYSTEM, NOR THE WASHINGTON STATE REFERENCE NETWORK.

VERTICAL DATUM

NAVD 88 BASED ON RTK GPS MEASUREMENTS CONST

SURVEY NOTES

- 1. DATA FOR THIS SURVEY WAS GATHERED BY FI ACCURACY REQUIREMENTS CONTAINED IN W.A.C BEEN MAINTAINED IN ACCORDANCE WITH MANU
- THIS MAP GRAPHICALLY REPRESENTS CONDITION PERFORMED DURING <u>DECEMBER OF 2021</u>.
- 3. THE CERTIFICATION OF THIS SURVEY AND MAP SPECIFICALLY DESIGNED TO MEET THEIR STATED FOR ANY ALTERNATIVE USE OF THIS MAP WITH
- 4. THE PURPOSE OF THIS SURVEY IS TO PROVIDE #6666800250 FOR PLANNING, DESIGN AND CO
- 5. UTILITIES OTHER THAN SHOWN MAY EXIST ON T SHOWN ARE IN THE EXACT LOCATION INDICATED CANNOT BE ACCURATELY, COMPLETELY, AND RE REQUIRED, THE CLIENT IS ADVISED THAT EXCA AS ACCURATELY AS POSSIBLE FROM FIELD SU
- 6. PARCEL AREA: 19,304 ± SQ.FT. (0.44 ACRES)
- 7. ALL DISTANCES AND DIMENSIONS SHOWN ARE
- 8. CONTOUR INTERVALS ARE 2-FOOT AND ARE SURVEY UTILIZING ELECTRONIC DATA COLLECTION
- 9. THE PROPERTY AND RIGHT-OF-WAY LINES SHO MONUMENTS, FROM WHICH WE CONDUCTED A M. RECORDED PLAT MAP. NO PROPERTY CORNERS
- 10. WE HAVE USED GRAPHIC SYMBOLS TO REPRESE DEFAULT SIZE OF THOSE SYMBOLS MAY NOT R

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		DATE	12/17/21	
		NOILAI	NOILEN	
		DESCR	OPE INFORM	
		NON	STEEP SL	
). 20200410000015 I VOLUME 78 OF PLATS, F	PAGES 29 AND 30, RECORDS OF KING COUNTY.	JEVIS	ADDED 3	
OF KING, STATE OF WASH	INGTON.	L.		
ORTH ZONE (NAD 83/201	1) BASED ON RTK GPS MEASUREMENTS CONSTRAINED TO	EV NO	-	
		<u>"</u>		
strained to the Washin	GTON STATE REFERENCE NETWORK.			
				ing 3 -7479 -0599
FIELD TRAVERSE UTILIZING .C. 332.130.090. ALL MEA UFACTURER'S SPECIFICATI	ELECTRONIC DATA COLLECTION, AND MEETS OR EXCEEDS SURING INSTRUMENTS EMPLOYED IN THIS SURVEY HAVE ONS.			CCT 4, Suit 98409- 3) 473
ons and features exist	TING AT THE TIME OF THIS SURVEY ONLY, WHICH WAS			ngin stree ngton : (253
P IS EXCLUSIVE TO THE N ED NEED(S). THAT CERTIF	IAMED CLIENT WHO REQUESTED THIS SURVEY. IT WAS ICATION DOES NOT EXTEND TO ANY OTHER PARTIES OR IRTELECATION BY THE SURVEYOR NAMING THOSE PARTIES			h 35th Washir
E A TOPOGRAPHIC MAP (ONSTRUCTION.	OF THE EXISTING CONDITIONS WITHIN KING COUNTY PARCEL			1 Sout coma,
THE SITE. THE SURVEYOR TED. LACKING EXCAVATION RELIABLY DEPICTED. WHEF AVATION MAY BE NECESS. URVEY INFORMATION.	R DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES , THE EXACT LOCATION OF UNDERGROUND FEATURES RE ADDITIONAL OR MORE DETAILED INFORMATION IS ARY. THE SURVEYOR DOES CERTIFY THAT THEY ARE SHOWN			260) Ta Ta
COMPUTER GENERATED FR	ROM GROUND FIELD TOPOGRAPHY GATHERED FOR THIS			
HOWN HEREON ARE BASED MATHEMATICAL CALCULAT	O ON FIELD TIES TO SEVERAL OF THE ORIGINAL PLAT TON OF THE PARCEL BASED ON THE GEOMETRY OF THE			
S WERE ESTABLISHED DU	THIS SURVEY. THIS MAP, SUCH AS UTILITIES, TREES AND FENCES. THE			
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TPN	TAX PARCEL NUMBER		С С	ACE ACE
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	- RIGHT OF WAY LINE		õ	Z B
	- ROAD CENTERLINE		Ğ	
	DECIDIOUS TREE (DIAMETER AS NOTED)		F	
the second se	MAPLE TREE (DIAMETER AS NOTED)			
0	PILING			. 2
o	WOOD FENCE			
	SPLIT RAIL FENCE			
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Ψ	TELEPHONE RISER	E		, ,
	GAS METER	E		0
Ш	WATER VALVE	D.	ATE SEALEI	12/20/2021
Ki Li	IRRIGATION CONTROL VALVE			PAD
0003	ROCKERY		ANRI OF	ASI CAL
	CONCRETE SURFACE GRAVEL SURFACE		PLS PROVIDENT	9286 STERIE SHIT
		Pf	ROJECT MAN	IAGER (AP
		D	-Sigin Rawn	M
				24 N _R 5 E
		FI	E NO _35970)
	SURVEYOR'S CERTIFICATE	D	ATE 12/20/2	2021

SURVEYOR'S CERTIFICATE I HEREBY CERTIFY THAT THIS MAP CORRECTLY REPRESENTS A TOPOGRAPHIC SURVEY MADE BY ME OR UNDER MY DIRECTION AND TO THE BEST OF MY KNOWLEDGE REPRESENTS THE TOPOGRAPHIC FEATURES AS THEY EXIST ON THE GROUND AS OF 12/3/2021. 12/20/2021 DATE KURT A! PARCHER P.L.S. NO. 49286

SHEET 1 OF 1 FILE NO 35970 CAPEX ENGINEERING LLC 2021

SCALE _1" = 20'